



**ANALISIS PORTAL BETON BERTULANG PADA STRUKTUR
GEDUNG BERTINGKAT BANYAK DENGAN TINGKAT
DAKTILITAS PENUH DAN ELASTIK PENUH**

SKRIPSI

diajukan guna melengkapi tugas akhir dan memenuhi salah satu syarat
untuk menyelesaikan Program Studi Ilmu Teknik Sipil (S1)
dan mencapai gelar Sarjana Teknik

Oleh

HERU NURCAHYO
NIM 021910301046

JURUSAN TEKNIK SIPIL
FAKULTAS TEKNIK
UNIVERSITAS JEMBER
2008

SUMMARY

Reinforcement concrete portal analysis in multistoried buildings structure with completed ductility level and completed elasticity

In multistoried building design, we have known about Partial Ductility and Completed Ductility, beside a level of completed elastic of them, there is a different of them, the main different is Deviation of ductility factor. In general, the earthquake proof building designed by the application of ductility concept. The Completed of Ductility of structure level has the structure that is able to face the deviation after elastic. This happen when the conditions at the maximum collapse that the ductility's factor is 5,3. The Completed Ductility design is usually difficult for realization because of reinforcement detailing. It is contrast with the completed elastic. This do not need with special detailing, because the completed elastic designs the building do not allowed to be deviation after elastic.

The purpose of observation knows about the different of structure design between the structure that designed by the completed ductility and completed elastic in multistoried building.

The regular of building structure's is able to be the nominal earthquake load that caused by the influence of the prediction of earthquake loads in each main coordinate of structure sketch, this is a total design lateral force at the base. SNI 1726 arranges the earthquake load static equivalent (V). Beside the building category has an important factor (I). The main coordinate of structure sketch and the way of earthquake loads has a strength reduction factor of earthquake (R) and elastic fundamental period of vibration (T1), so the total design lateral force at the base storied is able to count by equation:

$$V = \frac{C_1 I}{R} W_t$$

Where:

C₁ = Seismic coefficient it's given by response spectrum

I = Importance factor

W_t = the total seismic dead load and live load

R = strength reduction factor

V = the total design lateral force or shear at the base

R = 5,5 for completed ductility design and R = 1,6 for completed of elastic level, and the building is in the seismic zone intermediate.

This result got by area of reinforcement. Then it is tried to make a changing (alternative design); the design of a cross section beams and different column for each storied, it use a completed ductility and it concluded that the elected of different

design of across section structures element is effective. It can reduce the reinforcement using is 34, 8 %

The building of earthquake proof design is better to give more priority in material quality because the high quality of material could reduce the determining of across section design of structures element. So it would reduce the shear force at each storied.



DAFTAR ISI

	Halaman
HALAMAN JUDUL	i
RINGKASAN	vi
DAFTAR ISI	ix
DAFTAR TABEL	xiv
DAFTAR GAMBAR	xx
DAFTAR LAMPIRAN	xxi

BAB I PENDAHULUAN

1.1. Latar Belakang	1
1.2. Perumusan Masalah	2
1.3. Batasan Masalah	2
1.4. Tujuan dan Manfaat	3

BAB II TINJAUAN PUSTAKA

2.1. Pembagian Jenis Struktur	4
2.2. Daktilitas	4
2.3. Perencanaan Kapasitas	5
2.4. Beban Geser Dasar Nominal Statik Ekuivalen (V)	10
2.5. Faktor Keutamaan Penghunian (I)	11
2.6. Faktor Reduksi Gempa (R)	12
2.6.1 Faktor Daktilitas Struktur Bangunan	12
2.6.2 Faktor Reduksi Gempa	13
2.7. Waktu Getar Alami Fundamental (T1)	16
2.7.1 Persamaan Empiris Untuk T1	16
2.7.2 Pembatasan Waktu Getar Alami Fundamental	16
2.7.3 Rumus Rayleigh Untuk T1	17

2.8. Faktor Respons Gempa (C).....	18
2.9. Distribusi Gaya Lateral (Fi).....	20
2.10. Pembatasan Drift	20
2.10.1 Kinerja Batas Layan (Δ_s).....	20
2.10.2 Kinerja Batas Ultimit (Δ_m)	21
2.11. Keandalan dan Keamanan Struktur.....	22
2.12. Asumsi – asumsi yang digunakan pada penampang lentur	23
2.13. Desain lentur penampang balok beton bertulang	25
2.14. Desain tulangan geser penampang balok beton bertulang	22
2.15. Desain lentur dan geser pada kolom	26
2.15.1 Kolom dengan pengaku (Tidak bergoyang).....	27
2.15.2 Kolom tanpa pengaku (Bergoyang).....	29
2.15.3 Penulangan kolom.....	30
2.15.4 Perhitungan Tulangan Lateral Kolom.....	31
2.16. Desain dengan sap 2000	31
2.16.1 Pengertian SAP 2000.....	31
2.16.2 Sistem koordinat.....	31
2.16.3. Massa.....	33
2.16.4 Beban pada struktur.....	33
2.16.5 Derajat Kebebasan (DOF).....	37
2.16.6 Analisis Statik.....	39
2.16.7 Output Gaya – gaya Dalam.....	40

BAB III METODOLOGI PENELITIAN

3.1. Pengumpulan Data.....	42
3.2. Studi Literatur.....	42
3.3. Kerangka Penelitian	43
3.4. Flowchart Penelitia	44

3.5. Waktu dan Tempat Kegiatan	46
3.5.1 Tempat Kegiatan	46
3.5.2 Waktu kegiatan	46
 BAB IV HASIL DAN PEMBAHASAN	
4.1. Data Perencanaan Untuk Model Desain Awal	47
4.1.1 Gambar Model Desain awal.....	48
4.2. Analisa Beban Gempa	49
4.2.1 Perhitungan Beban Mati (Wt)	49
4.2.2 Taksiran waktu getar alami (T)	52
4.2.3 Mendefinisikan momen inersia dan kekakuan elemen portal	55
4.2.4 Analisa Terhadap Trayleigh.....	63
4.2.5 Analisa Struktur	67
4.2.6 Kinerja Batas Layan (Δs) dan Kinerja Batas Ultimit (Δm)	71
4.3. Analisa Beban Gravitasi.....	73
4.3.1 Analisa Beban	73
4.3.2 Hasil analisis beban gravitasi	77
4.4. Kombinasi Beban.....	82
4.5. Desain penulangan lentur balok untuk perencanaan daktilitas penuh... 91	
4.5.1 Perhitungan tulangan lentur balok portal arah X	91
4.5.2 Perhitungan tulangan lentur balok portal arah Y	97
4.5.3 Perhitungan tulangan lentur balok anak.....	102
4.5.4 Persyaratan komponen lentur sistem rangka pemikul momen	108
4.6. Desain tulangan geser pada balok tingkat daktilitas penuh	109
4.6.1 Desain tulangan geser portal arah X	109
4.6.2 Desain tulangan geser portal arah Y.....	112
4.6.3 Desain tulangan geser balok anak	115
4.7. Desain Penulangan kolom Untuk Perencanaan Daktilitas penuh..... 119	
4.7.1 Data Perhitungan	119

4.7.2 Perhitungan Gaya-gaya dalam.....	119
4.7.3 Perhitungan Luas Tulangan.....	122
4.8. Desain penulangan Lentur balok untuk perencanaan Elastik penuh.....	125
4.8.1 Perhitungan tulangan lentur balok portal arah X.....	125
4.8.2 Perhitungan tulangan lentur balok portal arah Y	131
4.8.3 Perhitungan tulangan lentur balok anak.....	137
4.9. Desain tulangan geser pada balok Perencanaan Tingkat Elastik Penuh..	143
4.9.1 Desain tulangan geser portal arah X	143
4.9.2 Desain tulangan geser portal arah Y.....	147
4.9.3 Desain tulangan geser balok anak	150
4.10. Desain Penulangan kolom Perencanaan Tingkat Elastik Penuh.....	153
4.10.1 Data Perhitungan	153
4.10.2 Perhitungan Gaya-gaya dalam.....	153
4.10.3 Perhitungan Luas Tulangan.....	156
4.11. Perencanaan Daktilitas Untuk Model Desain Alternatif.....	159
4.11.1 Data perencanaan model desain alternatif.....	159
4.11.2 Gambar model desain alternatif.....	160
4.11.3 Analisa beban gempa	161
4.11.4 Taksiran waktu getar alami (T).....	162
4.11.5 Analisa Terhadap Trayleigh.....	165
4.11.6 Analisa Struktur	167
4.11.7 Kinerja Batas Layan (Δs) dan Kinerja Batas Ultimit (Δm).....	167
4.11.8 Analisis beban gravitasi	170
4.12. Desain penulangan balok untuk perencanaan daktilitas penuh pada model desain alternatif.....	174
4.12.1 Perhitungan tulangan lentur balok portal arah X.....	174
4.12.2 Perhitungan tulangan lentur balok portal arah Y.....	180
4.12.3 Perhitungan tulangan lentur balok anak portal arah Y.....	186

4.13. Desain tulangan geser pada balok sistem rangka pemikul momen (SRPM) untuk tingkat daktilitas penuh.....	192
4.13.1 Desain tulangan geser portal arah X	192
4.13.2 Desain tulangan geser portal arah Y.....	195
4.13.3 Desain tulangan geser balok anak	198
4.14. Desain Penulangan kolom Perencanaan Tingkat daktilitas Penuh.....	202
4.14.1 Data Perhitungan	202
4.14.2 Perhitungan Gaya-gaya dalam.....	202
4.14.3 Perhitungan Luas Tulangan.....	205
4.15. Perbandingan Hasil Perhitungan Tulangan Lentur Balok dan Kolom..	209
 BAB V KESIMPULAN DAN SARAN	
5.1. Kesimpulan	217
5.2. Saran	217
 DAFTAR PUSTAKA	218
LAMPIRAN	